

ANALYSIS OF MULTISTORY STRUCTURE ON INCLINED PLANE

Aariz Quadar Pasha¹, Prof. Vikrant Dubey², Prof. Kapil Soni³ ¹Scholar M.Tech (Structural Engineering), ²Asst. Prof., ³HoD, Department of Civil Engineering, University Institute of Technology, RNTU Bhopal (M.P).

ABSTRACT: Earthquakes are very disastrous and cause a great harm to living life, material life and buildings. Hence proper dynamic analysis for building having earthquake threat is needed. This will ensure proper designs resulting in an earthquake proof structure. Different dynamic investigation techniques are accessible for tremor examination of multi-storey structures which are response spectrum method (RSM), seismic coefficient method (SCM), time history strategy and Codal provision technique (CPT). The analysis of a G+4 storey RCC building on varying slope angles i.e., 00, 150, 200, 250, 300 and 400 is studied and compared with the same on the flat ground. The structural analysis software STAAD Pro v8i is used to study the effect of inclined ground on building performance. The analysis is carried out to evaluate the effect of inclined ground on structural forces. Soil structure interaction must be suitably considered from design point of view. Overall displacement of the structure with respect to different inclined ground configurations is also analysed.

Keywords: Structural Behaviour, Horizontal Force, Bending Moment, Axial force, Slope Ground

I. INTRODUCTION

Now days, rapid construction is taking place in hilly areas due to scarcity of plain ground. As a result the hilly areas have marked effect on the buildings in terms of style, material and method of construction leading to popularity of structures in hilly regions. Due to inclined profile, the various levels of such structures step back towards the hill slope and may also have setback also at the same time. These structures become highly uneven and asymmetric, due to variation in mass and stiffness distributions on different vertical axis at each floor. Such construction in earthquake prone areas makes them to attract greater shear forces and torsion compared to normal construction.

A scarcity of plain ground in hilly area compels the construction activity on inclined ground. Slope construction of buildings constructed in masonry with mud mortar/cement mortar without conforming to seismic codal provisions and wind analysis have proved unsafe and, resulted in loss of life and property when subjected to earthquake ground motions and neglecting the winds which flows at upper part of the hills. Hilly areas are more prone in seismic activity and wind activity for e.g. Northern region of India.

II. OBJECTIVE OF THIS STUDY

Safety and serviceability is the prime requirement for buildings on inclined ground. To meet these requirements, the structure should have adequate lateral strength, lateral stiffness, and sufficient ductility. The main objective of present work is to investigate the structural behaviour of

building frame resting on inclined ground subjected to seismic loading. The objectives of this study are as follows:

- Analysis of inclined ground, its displacement which shows the behaviour as of regular building.
-Step Back Configuration
-Step Back and Set Back Configuration -Set Back Configuration
- Evaluating the displacements value as the slopes increases.
- Three dimensional space frame analysis is carried out for four different configurations of buildings ranging from G+ 4 storey resting on inclined ground under the action of seismic load by using STAAD software.
- Dynamic response of these buildings, in terms of base shear, fundamental time period and displacement will be presented, and comparison will be done by considering configuration as well as with other configurations.
- A suitable configuration of building to be used in hilly area is suggested.

METHOD OF ANALYSIS

- The analysis of multi-storey buildings for the gravity loads or vertical loads and horizontal loads can be done by analysis tool STAAD pro.
- For the static and dynamic analysis of multi-storey buildings have moment resisting frame By STAAD Pro. software Method.
- Equivalent static lateral force method – For Static analysis only discussed as per IS 1893(part-1):2002 for regular buildings only.
- Analysis of the soil bearing capacity of different hilly areas.
- Analysis of Displacement of Column in various slopes configurations.
- Comparisons of the structure of different configurations on the basis of angle of inclined with the help of STAAD-Pro.
- Graphical comparison will be done in accordance to the proposed structure and their different configurations.

III. METHODOLOGY

This thesis deals with relative study of behaviour of inclined ground building frames considering different inclination (00, 100, 150, 200, 250 and 300) under earthquake forces. The comparison of inclined ground and plane ground building under seismic forces is done. Here G+ 4storey is taken and same live load is applied in three the buildings for its

behaviour and comparison. The framed buildings are subjected to vibrations because of earthquake and therefore seismic analysis is essential for these building frames. The rigid system is determined by applying in five building frames in seismic intensities: V with the help of STAAD Pro.

IV. STRUCTURAL MODELS

Structural models for different inclined ground are shown in plan, elevation and 3D structural model of plane and inclined ground structures in Fig.

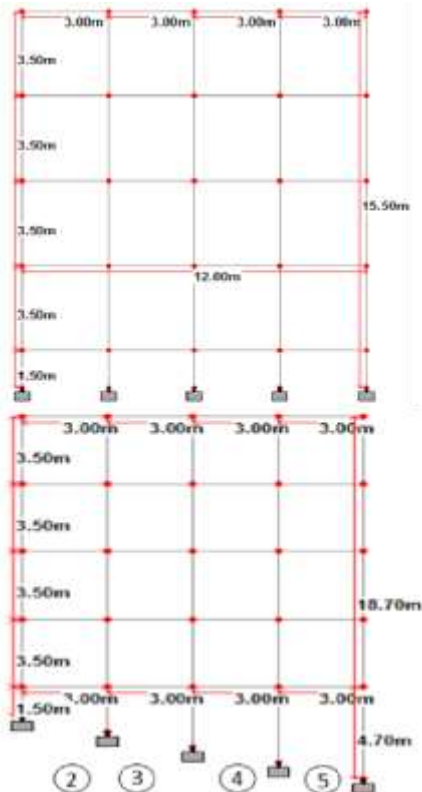


Fig 1 ELEVATION OF A INCLINED GROUND (0°)
 Fig 2 ELEVATION OF A INCLINED GROUND (15°)

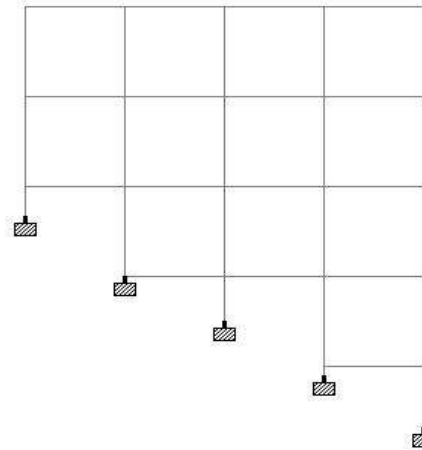
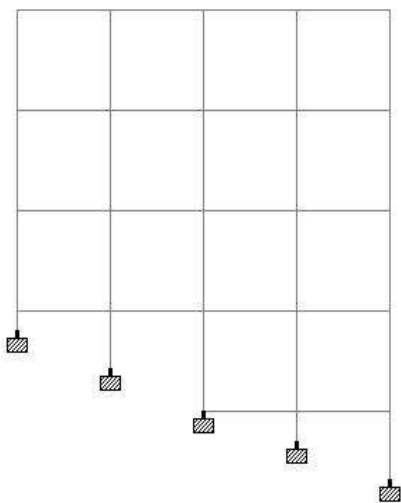


Fig 3 ELEVATION OF A INCLINED GROUND (20°)
 Fig 4 ELEVATION OF A INCLINED GROUND (30°)

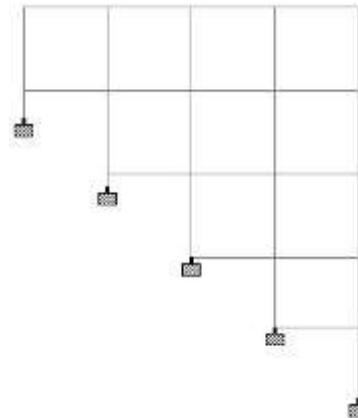


Fig 5 ELEVATION OF A INCLINED GROUND (40°)

ANALYSIS RESULT
 ANALYSIS RESULTS IN TERMS OF BENDING
 MOMENT MAXIMUM IN DIFFERENT CASES:

COLOUMN	BENDING MOMENT (KN-M)
1	146.4
2	163.7
3	177.8
4	158.3
5	155.5

Table-1 Bending Moment for 0°

COLOUMN	BENDING MOMENT (KN-M)
1	128.1
2	152.7
3	177.7
4	192.8
5	247.8

Table-2 Bending Moment for 15°

COLOUMN	BENDING MOMENT (KN-M)
1	125.3
2	149.6
3	172.9
4	189.65
5	201.22

Table-3 Bending Moment for 20°

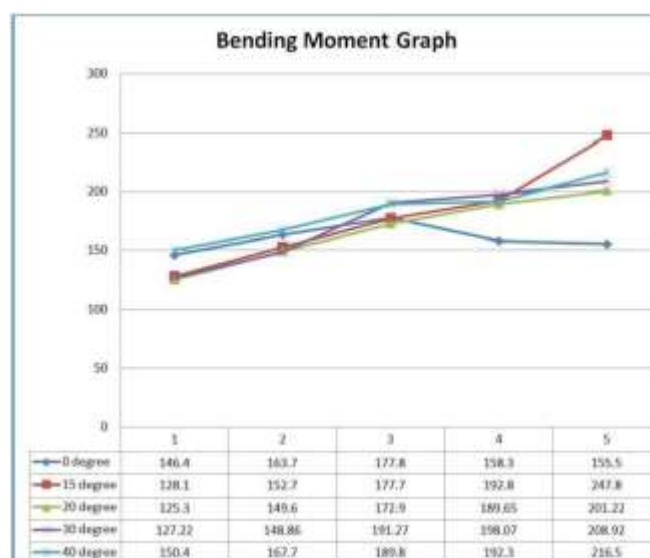
COLOUMN	BENDING MOMENT (KN-M)
1	127.22
2	148.86
3	191.27
4	198.07
5	208.92

Table-4 Bending Moment for 30°

COLOUMN	BENDING MOMENT (KN-M)
1	150.4
2	167.7

3	189.8
4	192.3
5	216.5

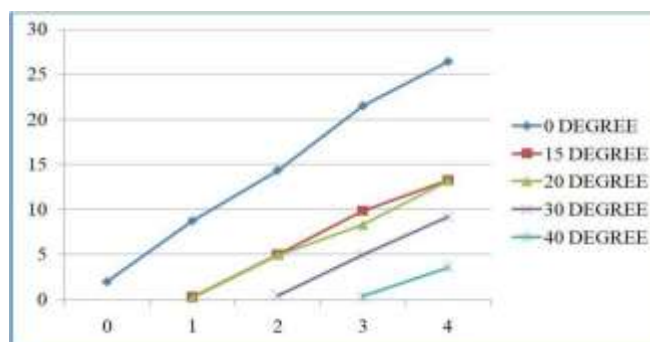
Table-5 Bending Moment for 40°



ANALYSIS RESULTS IN TERMS OF MAXIMUM DEFLECTION IN DIFFERENT CASES:

COLUMN NO.	FLOOR NO.	0	15	20	30	40
1	0	1.992	-	-	-	-
1	1	8.756	0.307	0.207	-	-
1	2	14.34	5.02	4.956	0.461	-
1	3	21.5	9.81	8.3	4.966	0.35
1	4	26.4	13.23	13.19	9.158	3.555

Table-6 MAXIMUM DEFLECTION FOR COLOUMN 1



COLUM N NO.	FLOO R NO.	0	15	20	30	40	COLUM N NO.	FLOO R NO.	0	15	20	30	40
2	0	1.95 9	2.3	1.88	2.21	1	4	0	1.88	2.67 3	2.3	1.8 8	2.21 6
2	1	9.70	10.4 4	8.68	6.34	5.16 2	4	1	8.08	11.2 2	10.4 4	8.6 8	6.34
2	2	17.0 5	19.4 2	16.4 0	12.7 4	10.1 2	4	2	16.06	22.1	19.4 2	16. 4	12.7 5
2	3	22.1 2	27.3 9	23.5 1	18.9 1	14.4 8	4	3	22.34 6	29.3	27.4	23. 5	18.9 2
2	4	27.6	33.3 4	28.9 6	23.7 6	17.7 6	4	4	27.33	36.8	33.3 4	29	23.7 7

Table-7 MAXIMUM DEFLECTION FOR COLOUMN 2

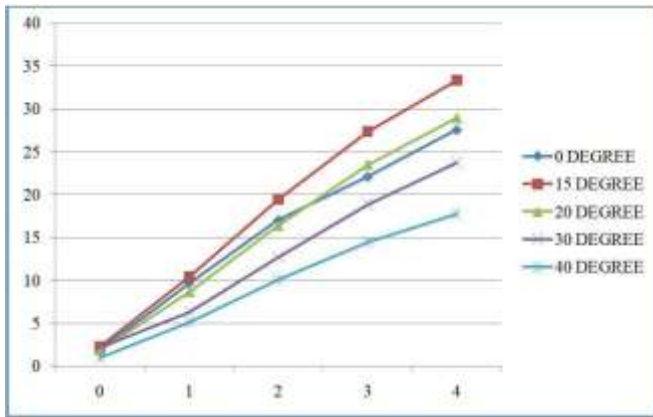
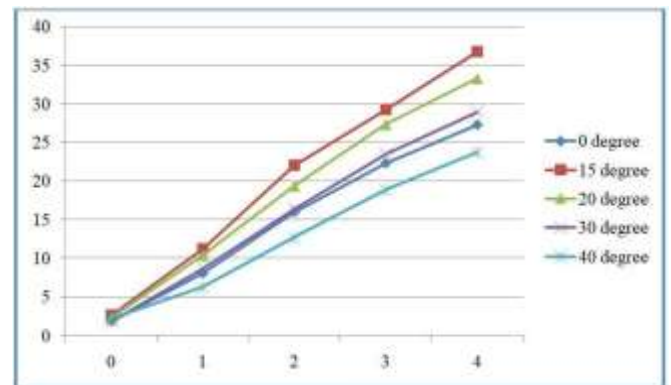
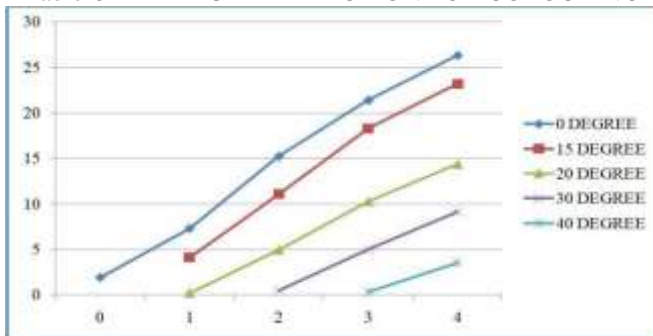


Table-9 MAXIMUM DEFLECTION FOR COLOUMN 4



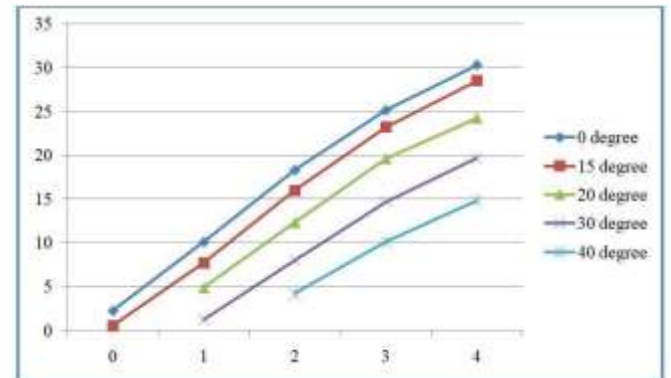
COLUM N NO.	FLOO R NO.	0	15	20	30	40
3	0	1.912	-	-	-	-
3	1	7.311	4.14	0.2 1	-	-
3	2	15.23 1	11.1 2	4.9 6	0.46 1	-
3	3	21.40 8	18.3 1	10. 3	4.99 6	0.3 5
3	4	26.33	23.2 3	14. 4	9.15 8	3.5 6

Table-8 MAXIMUM DEFLECTION FOR COLOUMN 3



COLUM N NO.	FLOO R NO.	0	15	20	30	40
5	0	2.28 9	0.55 3	-	-	-
5	1	10.1 4	7.72	4.91 5	1.29 9	-
5	2	18.3 3	16.0 2	12.3 5	8.04 2	4.25
5	3	25.1 6	23.2 3	19.6 1	14.7	10.1 3
5	4	30.3 2	28.5 2	24.2 4	19.7	14.8 7

Table-10 MAXIMUM DEFLECTION FOR COLOUMN 5



ANALYSIS OF SHEAR FORCE IN DIFFERENT CASES:

COLOUMN	SHEAR FORCE (KN)
1	64.7
2	59.3
3	71.5
4	76.6
5	82.2

Table-11 SHEAR FORCE FOR 0 DEGREE

COLOUMN	SHEAR FORCE (KN)
1	52.1
2	44.8
3	47.3
4	49.6
5	50.1

Table-12 SHEAR FORCE FOR 15 DEGREE

COLOUMN	SHEAR FORCE (KN)
1	46.3
2	32.3
3	29.3
4	29.8
5	29.6

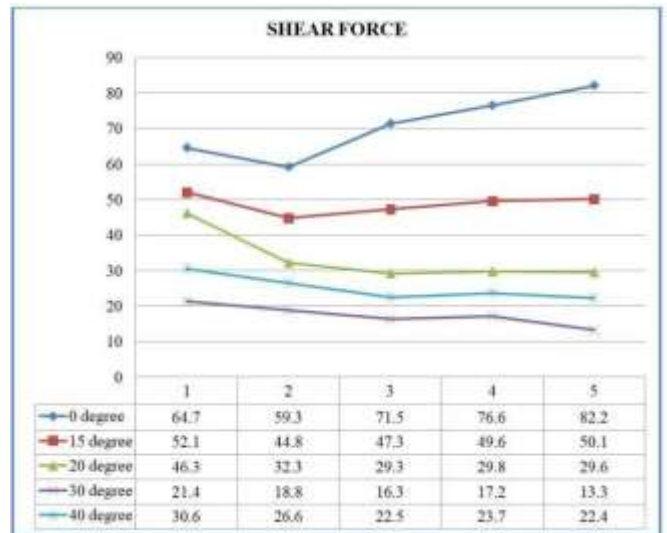
Table-13 SHEAR FORCE FOR 20 DEGREE

COLOUMN	SHEAR FORCE (KN)
1	21.4
2	18.8
3	16.3
4	17.2
5	13.3

Table-14 SHEAR FORCE FOR 30 DEGREE

COLOUMN	SHEAR FORCE (KN)
1	30.6
2	26.6
3	22.5
4	23.7
5	22.4

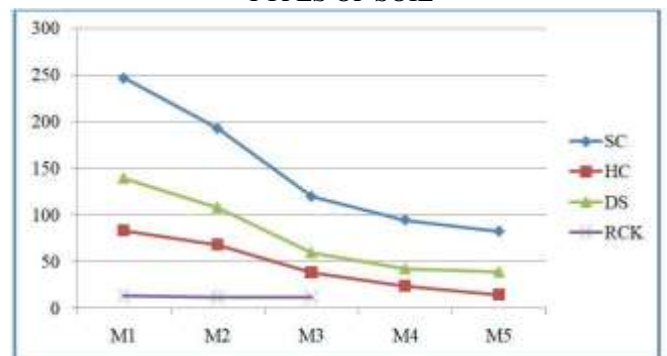
Table-15 SHEAR FORCE FOR 40 DEGREE



STOREY DISPLACEMENT WITH RESPECT TO DIFFERENT TYPE OF SOIL:

MODEL NUMBER	TYPE OF SOIL			
	SC	HC	DS	RCK
M1	247.10	83.48	139.61	13.66
M2	193.04	68.28	108.47	12.06
M3	120.17	38.54	59.90	12.22
M4	94.87	23.69	42.34	10.32
M5	82.66	14.42	39.06	9.76

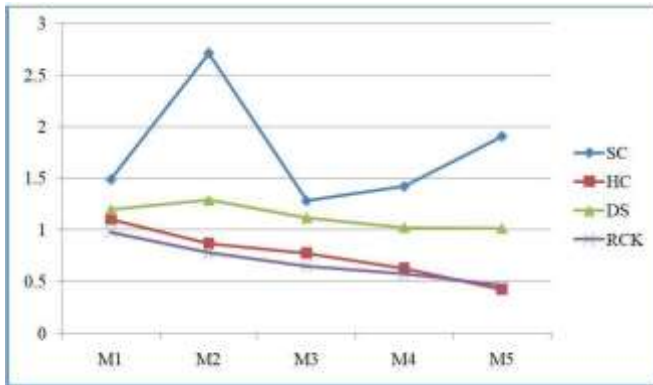
Table-14 STORY DISPLACEMENT FOR DIFFERENT TYPES OF SOIL



TIME PERIOD FOR DIFFERENT MODELS UNDER DIFFERENT SOIL CONDITIONS:

MODEL NUMBER	TYPE OF SOIL			
	SC	HC	DS	RCK
M1	1.493	1.105	1.201	0.982
M2	2.714	0.869	1.292	0.78
M3	1.285	0.777	1.120	0.653
M4	1.425	0.632	1.024	0.578
M5	1.910	0.426	1.017	0.467

Table-14 TIME PERIOD FOR DIFFERENT MODELS



V. CONCLUSION

Analysis of building frame for seismic forces is a common problem formulation now aday’s due to availability of several software tools and programs. Reinforced concrete (RC) structural frames are common form of constructions resting on plane and inclined ground (hilly areas) in India. There buildings are subjected to various types of forces during theirlifetime, such forces due to dead and live loads and dynamic forces due to the wind and earthquake. Results from seismic analyses performed on three RC buildings with three different ground slopes (0⁰, 10⁰, 15⁰, 20⁰, 25⁰ and 30⁰) have been carried out by using static method. The top storey displacement and the footing reaction, axial force, shear and moment action induced in columns and beams have been studied to investigate the influence of inclined ground onstructural performance of building frame.

The static analysis has been done on computer with the help of STAAD Pro. Software using the seismic parameters as per the IS: 1893- 2002 for the zone (IV) and the post processing result were obtained.

- Various codes are studied for analysis of seismic forces but every code as per time need to be reviesed and none of the code can be used for different area.
- In Step back buildings and Step back-Set back buildings, it is observed that extreme left column at ground level, which are short, are the worst affected. Special attention should be given to these columns in design and detailing.
- Although, the Setback buildings on plain ground attract less action forces as compared to Step back Set back buildings, overall economic cost involved in levelling the inclined ground and other related issues needs to be studied in detail.
- Bending moment in regular structure will be constant but in irregular structure where length of column changes according to slope then bending moment will vary respectively

REFERENCES

- [1] Montgomery CJ. “Influence of seismic effects on seismic design”, Canadian Journal of Civil Engineering 1981; 8: pp31-43.
- [2] IS 875(1987), Indian Standard Code of practice for Design loads for buildings andstructures, Bureau of Indian Standards, New Delhi. Ashraf Habibullah, Stephen Pyle, Practical three-dimensional non-linear static and dynamic analysis, Structure Magazine, winter, 1998 FEMA-356(2000), Pre standard and Commentary for the seismic Rehabilitation of buildings, American Society of Civil Engineers, USA.
- [3] Wilson, E.L., Eeri, M. and Habibullah, A. “Static and Dynamic Analysis of Multi Story Building Including P-Delta Effects”, Earthquake spectra, 3, 2 (1987).
- [4] Paul, D.K. “Simplified seismic analysis of framed buildings on hill slopes Bulletin of Indian Society of earthquake technology, Vol 30, No4, paper 335,Dec 1993, pp 113-124.
- [5] Paul, D.K. and Kumar, S. (1997) Stability Analysis of Slope with Building Loads.Soil Dynamics and Earthquake Engineering, 16, pp395-405.
- [6] Kumar, S., and Paul, D.K. (1998). A Simplified Method for Elastic Seismic Analysis of Hill Buildings.Journal of Earthquake Engineering 2 :(2), 241-266.
- [7] Satish Kumar and Paul. D.K., “Hill buildings configuration from seismic consideration”, Journal of structural Engg. Vol. 26, No.3, October 1999, pp. 179-185.
- [8] Government of Bureau of Indian Standards: IS-875, part 1 (1987), Dead Loads on Buildings and Structures, New Delhi, India.
- [9] Government of Bureau of Indian Standards: IS-875,

- part 2 (1987), Live Loads on Buildings and Structures, NewDelhi, India.
- [10] Tagel-Din Hatem, Kimiro Meguro., Applied Element Method for simulation of nonlinear materials: Theory and application for RC structures, Structural Eng. /Earthquake Eng., JSCE, Vol 17, No.2, 2000.
- [11] Vamvatsikos D., Cornell C.A. (2002).Incremental Dynamic Analysis. Earthquake Engineering and Structural Dynamics, 31(3): 491–514.
- [12] BIS. (2002). IS 1893 (Part 1) Indian Standard criteria for Earthquake Resistant Design of structures, Part 1: General Provisions and buildings (Fifth Revision). New Delhi, Bureau of Indian Standards.
- [13] IS 1893 (Part-I) 2002: Criteria for Earthquake Resistant Design of Structures, Part-IGeneral Provisions and Buildings, Fifth Revision, Bureau of Indian Standards, New Delhi.
- [14] Nalawade S.S. “Seismic Analysis of Buildings on Inclined Ground,” M.E. Dissertation, University of Pune, Pune Dec-2003.
- [15] Bozorgnia Y, Bertero V, "Earthquake Engineering: From Engineering Seismology to Performance-Based Engineering", CRC Press, 2004.
- [16] Birajdar B.G., Nalawade. S.S., 13WCEE 2004 Seismic analysis of buildings resting on inclined ground. Conference on Our World in Concrete & Structures: 29 – 30 August 2002, Singapore.
- [17] Birajdar B. G., and Nalawade S. S. 2004.Seismic Analysis of Buildings Resting on Inclined Ground.In Thirteenth World Conference on Earthquake Engineering (13WCEE).Vancouver, Canada, Paper No.1472.
- [18] Hajra B and Godbole P.N. (2006). “Along Seismic Load on Tall Buildings Indian Codal Provisions.” 3NCWE06 Kolkata, pp 285-292.
- [19] Agarwal P. and Shrikhande M. 2006, Earthquake resistant design of structures (Prentice-Hall of India Private Limited, New Delhi, India) Applied Technology Council (1996): Seismic Evaluation and Retrofit of Concrete Buildings, ATC-40, Vol. 1.
- [20] Lee Han Seon ,Dong Woo Kee, 2007, Seismic response characteristics of high-rise RC wall buildings having different irregularities in lower stories with inclined ground, Engineering Structures 29 (2007):3149–3167.
- [21] Athanassiadou C.J, 2008, Seismic performance of R/C plane frames irregular in elevation, Engineering Structures 30 (2008):1250–1261.
- [22] Emraherduran (2008), “Assessment of current nonlinear static procedures on the estimation of torsional effects in low-rise frame buildings in inclined ground”, Engineering Structures 30 (2008): 2548-2558.
- [23] Guan Y.H., et al. (2011) Studied on the Earthquake Disaster Reduction Information Management System and Its Application. International Journal of Intelligent Systems and Applications, Vol-1, pp 51-57.
- [24] Gourabi, A. and Yamani, M. (2011) Active Faulting and Quaternary Landforms Deformation Related to the Nain Fault on inclined ground. American Journal of Environmental Sciences, 7, 441-447.
- [25] Vijaya Narayanan A.R., Rupen Goswami and Murty C.V.R., 15WCEE 2012 Performance of RC Buildings along Hill Slopes of Himalayas during 2011 Sikkim Earthquake.
- [26] Pandey A.D, Prabhat Kumar, Sharad Sharma., International Journal of Civil and Structural Engineering Vol. 2, No. 2, 2011 Seismic soil-structure interaction of buildings on hill slopes.